

# Study on Impact Strength of Concrete by Using Basalt Fibre

P.Manibalan<sup>1</sup>, R.Baskar<sup>2</sup>,

Research Scholar, Dept of Structural Engineering, Annamalai University, Tamilnadu, India<sup>1</sup>.  
Associate Professor, Dept Of Structural Engineering, Annamalai University, Tamil Nadu, India<sup>2</sup>.

**Abstract:-** This paper presented an experimental investigation on the impact failure energy of high strength concrete by basalt fiber. Basalt fibers are relatively cheaper and new fibers for concrete which are investigated by a few researchers. Two different grade (M40 and M50) of concrete were used as the basalt fiber in various volume fractions such as 0.3%, 0.6%, 0.9% and 1%. The samples were tested under instrumented falling impact loading and compared with conventionally cured ones. The results indicated that the concrete containing 0.9% volume fraction of fiber gave the best performance under impact loading.

## I. INTRODUCTION

Concrete is one of the most conventionally consumed construction materials. Concrete has several advantages such as durability, formability and desired mechanical strength which gives it an edge over the other conventional building materials but it has few disadvantages such as low tensile strength and strain capacity. Due to high corrosion resistance, high ductility and sufficient durability, FRC is widely used especially in the military and marine fields, for instance, in fortified structures, blast resistant structures, offshore platforms, the exploitation of undersea oil engineering, etc.

Fiber inclusion in matrix greatly influences the properties of concrete and various studies have shown that the fibers can significantly improve the engineering properties of the concrete. Different types of fibers such as asbestos, cellulose, steel, polypropylene, PVA, carbon, basalt, aramid, polyethylene and glass have been used to reinforce cement products. Basalt fibre (BF), is a new kind of inorganic fibre extruded from melted basalt rock and is currently available commercially. The manufacturing process of this kind of fibre is similar to that of glass fibre, but with less energy consumed and no additives, which makes it cheaper than glass or carbon fibres.

It is known that the BF has better tensile strength than the E-glass fiber, greater failure strain than the carbon fiber as well as good resistance to chemical attack, impact load and fire with less poisonous fumes. So, BF has a potential to be a suitable replacement for glass, steel and carbon fibers in many construction applications.

Other advantages such as high modulus, heat resistance, good resistance to chemical attack, excellent interfacial shear strength and currently commercial availability, enable BF a good alternative to glass, carbon or aramidic fibre as a reinforcing material in concrete composite showed that the performance of dynamic modulus of elasticity and quality loss of BF concrete in freezing and thawing process is obviously better than the plain concrete. Although several types of fibers have been used in concrete, however there is only limited information available on mechanical properties and fracture behavior of high strength concrete incorporating BF which is of great importance in understanding the material behavior and in designing structures.

The main objective of this investigation is to study the effect of the impact behavior of basalt fibre concrete, compared with conventional concrete. The impact strength of basalt fibre concrete were tested and analyzed in this study.



Figure – 1 Basalt fibre

## II. EXPERIMENTAL STUDY

### A. Material and mixture proportion

Ordinary Portland Cement (OPC) 53 grade corresponding to ASTM Type I cement with a specific gravity of 3.15 was used in concrete mixtures. The coarse aggregates consisted of gravel with a maximum size of 15 mm. River sand was utilized as fine aggregates. The concrete specimens were demolded 24 h after casting and then subjected to curing at 95% relative humidity (RH) for 28 d. Afterward, the 28 d concrete strength was determined. The average concrete cubic compressive strength was  $f_{cu} = 48.25$  MPa, and 58.25 MPa the average value of axial compressive strength of three concrete specimens for M40 and M50 grade of concrete was

$f_c = 42.8 \text{ MPa}$  and  $53.8 \text{ MPa}$ . Four different volume fractions (0.3%, 0.6%, 0.9%, and 1%) of BF respectively, were adopted to study their effect on the properties of concrete. Detailed impact properties and the pictures of the basalt fiber used in this study are presented in Table 2, Table 3 and Fig. 1 respectively. The nomenclature adopted was: BF representing the basalt fiber reinforced concrete, respectively and the numbers written after BF letters indicating the percentage of fiber added in the concrete.

*B. Mixing and curing*

The mixing process started with the dry mixing of the coarse and fine aggregates for 1 min. Then, the cement was added and followed by the dry mixing for another 1 min. Further, fibres were added into the dry mixture for another 1 min. Finally, water was added slowly. The fresh concrete was mixed for 3 min to ensure even dispersion of fibres in the concrete. The fresh concrete was cast in  $150 \times 150 \times 150 \text{ mm}$  molds for impact strength test. In this study, every test result consists of the average of three replicate tests. After casting, specimens were cured at  $20^\circ\text{C}$  in molds covered by a polyethylene film to prevent moisture loss. Then, the specimens were de-molded after 24 h and were moved to saturated lime water at  $20^\circ\text{C}$  until the testing.

Table - 1. Mixing proportion for  $1 \text{ m}^3$

Mixture	W/B	Water Kg/m <sup>3</sup>	Cement Kg/m <sup>3</sup>	Fine Aggregate Kg/m <sup>3</sup>	Coarse Aggregate Kg/m <sup>3</sup>	Volume Fraction $V_f$	Fiber Kg/m <sup>3</sup>
M40 (0%)	0.40	160	400	660	1168	0	0
M40 (0.3%)	0.40	160	400	660	1168	0.3	7.9
M40 (0.6%)	0.40	160	400	660	1168	0.6	15.9
M40 (0.9%)	0.40	160	400	660	1168	0.9	23.8
M40 (1%)	0.40	160	400	660	1168	1	26.5
M50 (0%)	0.35	147.6	422	621	1284	0	0
M50 (0.3%)	0.35	147.6	422	621	1284	0.3	7.9
M50 (0.6%)	0.35	147.6	422	621	1284	0.6	15.9
M50 (0.9%)	0.35	147.6	422	621	1284	0.9	23.8
M50 (1%)	0.35	147.6	422	621	1284	1	26.5

*C. Impact Test*

The impact resistance of the specimens was determined in accordance with the procedure proposed by the ACI Committee 544.2R-89. For this purpose, from each batch, six specimens were used and the specimens were subjected to drop weight test. The impact load was applied with hammer onto a 5.180 kg ball of 60.2 mm diameter, dropped repeatedly from a 580 mm height on the center of the top surface of the specimens. The drop weight test arrangement was as shown in Figure 2. In each test, the number of blows (N1) required to produce the initiation of crack was recorded as the initial crack strength, and the number of blows (N2) needed to cause failure of the specimen was recorded as the failure strength; this method has been used by several researchers.

The energy absorption capacity of each specimen used in this test was calculated using Equation (1):

$$\text{Impact Energy } U = NmgH \dots\dots\dots(1)$$

Where,

- N = Number of blows
- m = Weight of the drop cylinder
- g = Loading due to acceleration
- H = Height of free fall.



**Figure -2** Drop weight impact test

### III. TEST RESULT AND DISCUSSION

The number of blows required to cause the first visible crack (N1) and final failure (N2) of concrete specimens are indexed in Table-2 and Table 3 and the impact energy corresponding to number of blows are shown in Figure-2. The impact energy of specimens during every blow can be calculated as follows. Substituting the corresponding values in following Equation.

Impact energy for initial failure =  $N1mgH$ .

Impact energy for ultimate failure =  $N2mgH$ .

Initial impact energy,  $U1 = 24 \times 5.180 \times 9.81 \times 0.58 = 707.36 \text{ Nm}$ .

Ultimate impact energy,  $U2 = 34 \times 5.180 \times 9.81 \times 0.58 = 1002.09 \text{ Nm}$ .

By adding 0.3%, 0.6%, 0.9% and 1.0% dosage of basalt fiber in M40 concrete the energy input necessary to cause the visibility of first crack was increased by 120%, 130%, 150% and 124%, respectively and the energy necessary to cause failure of concrete specimen was increased by 108%, 125%, 142% and 112% over the plain concrete specimen. Similarly for 0.3%, 0.6%, 0.9% and 1.0% dosage of basalt fibre in M50 concrete, the energy required to cause the initiation of first crack was increased by 121%, 134%, 150% and 131% respectively, and the energy required to cause failure of concrete specimen was increased by 125%, 137%, 154% and 139% over the plain concrete specimen. Hence it was observed that, increasing the volume fraction of basalt fiber increases the impact energy of concrete upto 0.9% significantly, in both the first crack stage as well as failure stage. This proves that the basalt fibers act as an effective crack arrestor in case of FRC, when an impact load is encountered. Thus the plain concrete exhibits an early brittle failure when compared to FRC which shows better ductile properties.

**Table- 2.** Impact Strength for M40 Grade Concrete

<b>Grade of concrete</b>	<b>Fiber content in %</b>	<b>Impact energy for initial failure (U1)</b>	<b>Impact energy for ultimate failure (U2)</b>
<b>M40</b>	0	1738.9 (59 blows)	2063.12 (70 blows)
<b>M40</b>	0.3	2092.59 (71 blows)	2239.96 (76 blows)
<b>M40</b>	0.6	2269.43 (77 blows)	2593.64 (88 blows)
<b>M40</b>	0.9	2623.11 (89 blows)	2934.69 (93 blows)
<b>M40</b>	1	2169.43 (73 blows)	2323.11 (79 blows)

Table-3. Impact Strength of M50 Grade Concrete.

Grade of concrete	Fiber content in %	Impact energy for initial failure (U1)	Impact energy for ultimate failure (U2)
M50	0	1797.86 (61 blows)	1886.28 (64 blows)
M50	0.3	2181.01 (74 blows)	2357.85 (80 blows)
M50	0.6	2416.80 (82 blows)	2593.64 (88 blows)
M50	0.9	2711.53 (92 blows)	2917.84 (99 blows)
M50	1	2357.85 (80 blows)	2623.11 (89 blows)

#### IV. CRACK PATTERN

The crack pattern for different proportions of fibre added is shown in fig 4. The plate in which 0% fibres were added brittle mode of failure was observed and it was broken into two pieces. Adding the fibre to concrete lead to encounter the ductile mode of failure and bridging the number of cracks which displays the beneficial effects of adding fibre to concrete.



Figure – 4. Crack Pattern

#### IV. CONCLUSION

The performance of fibre concrete plate under impact loads was very positive especially in basalt fibre reinforced concrete. Under impact loading, a ductile failure was observed in non fibrous concrete. The failure pattern of plate shows that incorporation of basalt fibre as an arrestor of crack propagation considerably improves the ability of concrete to absorb kinetic energy.

#### Reference

- Alavi Nia A, Hedayatian M, Mahmoud N and Afrough Sabet V. 2012. An experimental and numerical study on how steel and polypropylene fibers affect the impact resistance in fiber-reinforced concrete. *International Journal of Impact Engineering*. 46: 62-73.
- Andrea C, Roberto. 2010. Fracture behaviour of plain and fiber-reinforced concrete with different water content under mixed mode loading. *Materials and Design*. 31: 2032-2042.
- Angela G G, Kypros P, Kyriacos N and Maria Vania N N P. 2012. Fatigue resistance and cracking mechanism of concrete pavements reinforced with recycled steel fibres recovered from post-consumer tyres. *Engineering Structures*. 45: 385-395.
- Atef B, Ashraf F and Andrew K. 2006. Statistical variations in impact resistance of polypropylene fibre reinforced concrete. *International Journal of Impact Engineering*. 32: 1907-1920.
- Banthia N, Yan C and Saks K. 1998. Impact Resistance of Fiber Reinforced Concrete at Subnormal Temperatures. *Cement and Concrete Composites*. 20: 393-404.
- Carpinteri, A., Brighenti, R., 2010. Failure Behaviour of Plain and Fiber-reinforced Concrete with Different Water Content under Mixed Mode Loading. *Materials and Design*, 31:2032–2042.
- Chen Xiang-yu., Ding Yi-ning., Azevedo, C., 2011. Combined Effect of Steel Fibres and Steel Rebars on Impact Resistance of High Performance Concrete. *J. Cent South Univ Technol*, 18:1677–84.
- G. Murali, A. S. Santhi and G. Mohan Ganesh, 2014. Effect of Crimped and Hooked End Steel Fibres on the Impact Resistance of Concrete. *Journal of Applied Science and Engineering*, 17:259-266.
- G. Murali, A. S. Santhi and G. Mohan Ganesh, Empirical Relationship between the Impact Energy and Compressive Strength for Fibre Reinforced Concrete. *Journal of Scientific & Industrial Research*, 73: 469-473.

10. Ghavami, K., 2005. Bamboo as Reinforcement in Structural Concrete Elements. *J Cem Concrete Composite*, 27:637–649.
11. Gumble, E.J., 1963. Parameters in the Distribution of Fatigue Life. *Journal Eng Mech, ASCE*, (October) Jayatilaka, A., De, S., *Failure of Engineering Brittle Materials*, Applied Science, London, 1979.
12. Lu, X. and Hsu, C., 2006. Behavior of High Strength Concrete with and without Steel Fibre Reinforcement in Triaxial Compression, *Cement and Concrete Research*, 36:1679-1685.
13. Mahmoud, N., Afroughsabet, V., 2010. Combined Effect of Silica Fume and Steel Fibers on the Impact Resistance and Mechanical Properties of Concrete. *International Journal of Impact Engineering*, 37:879–886.
14. Mahmoud, N., Afroughsabet, V., 2010. The Effects of Silica fume and Polypropylene Fibers on the Impact Resistance and Mechanical Properties of Concrete. *Construction and Building Materials*, 24: 927–933.
15. Mohammadi, Y., Carkon-Azad, R., Singh, S. P. and Kaushik, S. K., 2009. Impact Resistance of Steel Fibrous Concrete Containing Fibres of Mixed Aspect Ratio, *Construction and Building Materials*, 23:183-189.
16. Mohammadi, Y., Singh, S.P., Kaushik, S.K., 2008. Properties of Steel Fibrous Concrete Containing Mixed Fibres in Fresh and Hardened State. *Construction and Building Materials*, 22: 956–65.
17. Nataraja, M. C., Dhang, N. and Gupta, A., 1999. Statistical Variations in Impact Resistance of Steel Fibre -Reinforced Concrete Subjected to Drop Weight Test, *Cement and Concrete Research*, 29:989-995.
18. Nataraja, M. C., Nagaraj, T. S. and Basavaraja, S. B., 2005. Reproportioning of Steel Fibre Reinforced Concrete Mixes and their Impact Resistance, *Cement and Concrete Research*, 35: 2350-2359.
19. Taner Yildirim, S., Cevdet, E. and Fehim Findik, E., 2010. Properties of Hybrid Fibre Reinforced Concrete under Repeated Impact Loads, *Russian Journal of Nondestructive Testing*, 46:538-546.